## Static Stress Analysis on the Large Capacity Thin-Wall UF<sub>6</sub> Storage Cylinder Intended To Be Used in Transport\*

*C.K. Chung, JL. Frazier, D.K. Kelley Lockheed Martin Energy Systems. Inc.* 

### INTRODUCTION

Large diameter ( 48-inch) cylinders are used for the storage and transport of depleted uranium hexafluoride (DUF<sub>6</sub>). This DUF<sub>6</sub> is the residual material from uranium enrichment operations from the three gaseous diffusion plants in the United States. Gaseous diffusion operations since the mid 1940s have produced an inventory of approximately 50,000 of these cylinders with nominal content of 14 tons  $DUF<sub>6</sub>$ .

The large diameter cylinders were originally designed and fabricated for temporary storage of  $DUF<sub>6</sub>$ . The cylinders were fabricated from pressure vessel grade steels according to the provisions of the *ASME Boiler and Pressure Vessel Codes* (Section VIII, Div. 1, 1992). These cylinders were constructed in thick wall (5/8-inch wall thickness) and thin-wall (5/16-inch wall thickness) versions.

These cylinders are currently stored in outdoor arrays in a two tier assembly with the contacts being shell to stiffener rings. The lower tier cylinders are supported by chocks. The cylinders are subjected to general and accelerated atmospheric corrosion and to possible damage from stacking and other handling operations.

Finite element stress analysis and fracture mechanics methodology have been utilized to assess cylinder structural integrity during handling operations and degradation of the cylinder shell resulting from corrosion or other damage mechanisms.

#### ASSUMPTIONS

The mass of the  $DUF<sub>6</sub>$  content is assumed in a line element, which coincides with the centerline of the cylinder. The centerline element distributes the lumped weight of the content on the selected nodal points of the shell by connection with weightless line

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elements. Each node on the centerline element connects 13 nodes on the shell which are in the plane of the lower half portion of the cylinder separated by 15 degrees.

The shell to stiffener ring contacts are each modeled with nine rigid links. They form an area of 1 inch (ring thickness) in the axial direction by 3/8 inch on the cylinder circumference.

The contact elements between the lifting lug and the stiffening ring are treated as a continuous body.

The cylinder pressure vessel grade steels of construction are assumed nominal values of  $30 \times 10^6$  lbs/in<sup>2</sup> and 0.3 for the modulus of elasticity and a Poisson's ratio, respectively.

## MODELING OF THE FINITE ELEMENT PROBLEMS

The cylinder assemblies are fabricated of 5/16-inch thick plates for the shell, three 2-1/2  $\times$ l-inch thick stiffener rings, and two 2: 1 ellipsoidal heads. Since the geometry and loadings are symmetrical with respect to a plane containing the centerline of the middle stiffener ring, a half of the cylinder is modeled for the analysis.

The right hand coordinate system of the computer model defines the direction of gravitational acceleration as negative in the Y -axis, and the centerline of the cylinder axis is in the X-axis. The static analyses are performed with the consistent mass model by the Engineering Mechanics Research Corporation (EMR.C), Display III of the NISA II program (1994).

The model has three-dimensional solid, shell, and beam elements. The centerline elements of the cylinder, beam elements, carry the entire content weight of the cylinder. The center line elements distribute the weight on the nodes of the shell elements by connecting bar elements from nodes on the shell elements to the centerline elements.

The shell and stiffener ring contacts are modeled with rigid links that express the deflection at one end as kinematically equivalent deflections of the other end. The wooden chocks are modeled both as rigid supports and as flexible supports.

## DISTRIBUTION OF THE CONTENT WEIGHT ON THE CYLINDER SHELL

The method of calculation of the density for the content,  $d_{bar}$  and for the shell,  $d_{shell}$ follows. Maximum volume of the content,  $V$ , for the type 48-G or 48-OM cylinder is 139.72 ft<sup>3</sup>. The equivalent length,  $L_e$ , of the cylinder with an inside diameter of 48 inches to contain a volume,  $V$ , is

 $V = 139.72 * 1728$  in.<sup>3</sup> = 241,436.16 in<sup>3</sup> =  $\pi * (r_i)^2 * L_e$ ,

therefore,

 $L_{\rm e}$  = 241,436.16/(3.14159 \* 24<sup>2</sup>) = 133.42288 in., and  $d_{shell}$  = 490/(1728 \* 32.174 \* 12) = 7.34456 \* 10<sup>-4</sup> lbf-s<sup>2</sup>/in.<sup>4</sup>.

The models have bar elements with cross sectional areas of 1.000 square inch to represent the  $UF_6$  content weight.

 $d_{bar}$  = 28,000 lbf/(cross sectional area  $* L$ ) = 28,000/(1.000  $*$  133.42288)  $= 0.54355$  lbf-sec<sup>2</sup>/in<sup>-4</sup>.

### STRESS ANALYSIS

Location of the Wooden Support Chocks Versus Maximum Stress for Single Tier Storage Cylinders. The stresses have been calculated for the shell thickness of 0.250 inch cylinders at different support locations of the wooden chocks. One wooden chock on the half cylinder has been modeled with rigid restraints and with flexible restraints with the spring rate of  $1.0 \times 10^6$  lbs/in. The summary of computations with the maximum von Mises-Hencky (M-H) stress versus the location of the chock is provided on Table 1 and plotted on Figure 1. It can be concluded that the location of the chock is better outside and close to the stiffener ring.

For the three computations at the top of Table 1, FS250H, FWS250, and FWR250, the chock locations are the same, but the type of support or fillet weld with the stiffener rings are different. The flexible support on the fillet-welded model has higher M-H stress, and the rigid supported model has lower M-H stress than that of the flexible one.



#### Table 1. Location of Chocks versus von Mises-Hencky Stress for Single-Tier Cylinders

The cylinder is supported by wooden chocks (7-inch width by 8.5 inches high). The negative sign denotes chock locations outside the ring; the positive sign indicates distances inside the ring (toward the center of the cylinder).

Maximum Stress Versus Location of the Stacked Cylinder. The two-tier stacking array forms an isosceles triangle with two 52.5 degree angles. Figure 2 represents the maximum M-H contact stress for the wall thickness of 0.250 inch for horizontal offset

## **Table B**



#### **Position of the Stacked Cylinder versus Stress**

(1) The horizontal offset distance between bottom cylinder and the top cylinder. The negative distance denotes a negative x-direction between the top and bottom cylinder.

(2) The bottom cylinder is supported by the wooden chocks (7" width by 8.5" high). It is assumed that the supporting region of the chock is 5.000" width saddle symmetrically with respect to the centerline of the width. A negative sign denotes that the centerline of the chock is outside (cylinder head side) from the centerline of the ring. A positive sign denotes the direction toward the center of the cylinder.

(\*) These points were used for the Figure 2.











Figure 3 von Mises-Hencky Stresses for Stacked Cylinders

distance between the bottom and the top cylinder. The maximum stress occurs at the contact point, and it is localized because of the thin shell. The maximum stress becomes higher if the offset distance is greater in the negative direction from the bottom cylinder.

The model with the fillet weld yields relatively lower contact stresses as the loading point is moved closer to the stiffener. The highest stress is at the contact points, and it is inversely proportional to the shell thickness. As an example, from Table 2 the stresses of the files S6L250 and S6L250FW (Figure 3) are 23.21 ksi and 21 .31 ksi, respectively; those of S25L25 and S25L25FW are 43.05 ksi and 16.03 ksi, respectively. The file names with "FW" denote that the model has a fillet weld of 0.250 inch on both sides of the stiffener. The file names with "S6L" and "S25L" represent distances of 6.000 and 1.250 inches, respectively, between the center lines of the bottom ring and top ring. The model S25L has a face to face distance of 0.250 inch between the top and the bottom cylinder stiffener.

Lifting of Breached Cylinders. Stresses have been analyzed for the lifting of four breached UF<sub>6</sub> cylinders. Lifts have been analyzed using lifting lugs, a crane with straps, and the Raygo Wagner cylinder carrier. The summary of load cases and stresses are provided in Table 3.

Cylinder stresses were computed by FEM. A uniform shell thickness of 0.250 inch was used for the calculations except for the thinned regions adjacent to the breached areas of the cylinders. The thinned region was modeled as a border around the breach having a thickness of the thinnest measurement reported as shown by "Min. Shell Thickness Measured" on Table 3. The maximum calculated stresses occur in the cylinder shell adjacent to the breached areas and are presented in Table 3. The load cases and computed stresses in Table 3 provide either a "recommended" or "not recommended" for a particular cylinder and lift condition. The maximum acceptable stress for a recommended lift condition is below material yield strength.

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**Inspection Criteria for 48-inch UF<sub>6</sub> Cylinders.** A standard for UF<sub>6</sub> cylinders has been established by American National Standard Institute (ANSI). This standard, ANSI N14.1 (1990) includes only those cylinders meeting all of the acceptance criteria for  $UF<sub>6</sub>$ handling. The defect criteria of ORO-651 distinguishes between accepTable 1nd unacceptable classes of damage, and these are classed for severity. This study considered unacceptable damage such as gouges or cuts and visible cracks.

The significance of gouges or cuts and cracks can be evaluated in terms of the localized effects on the stress fields resulting from internal pressurization and from mechanical loadings. These effects have been treated both by finite element analysis and by fracture mechanics techniques as shown in Appendix A. This study shows that such defects, which do not violate the 0.250-inch minimum wall thickness specified in ANSI N14.1 and OR0- 651, do not result in local stresses which exceed the ASME Section VIII, Division 1 allowable values.

For cylinders with 0.250-inch wall thickness, if a 1116-inch deep flaw or gauge is not adjacent to the stacking contact points the cylinder stress meets ASME Section VIII, Division 2 (1992) requirements.



## Table 3. Summary of the Cylinder Load Cases and Stresses

<sup>1</sup> Dimension  $x_1$  or  $x_2$  is measured from the origin of the coordinates (see Figure 1) to the close edge of the strap. <sup>2</sup> Measurements were obtained from the UF<sub>6</sub>-Cylinder Wall Thickness Report.

<sup>3</sup> A calculated equivalent uniaxial tensile stress adjacent to the breached area-Hencky von Mises stress.

' Estimated angle between the vertical axis and the strap/chain in the plane of the cylinder axis and vertical axis.

 $'$  Straps are placed symmetrically with respect to center of gravity except where  $x_1$  is specified.

• Recommends Raygo Wagner for rotating. if necessary but not for carrying the cylinder.

'For the alternate straps. if needed, 4-inch wide or 2·foot long straps can be used.

• This condition is to lift the plug end only to relocate the chock. It is to be used, if necessary, to achieve a vertical lift for the case C I.

## **CONCLUSION**

Eighty-eight percent of the thin-wall  $UF_6$  cylinders, types 480M and 48G at the Lockheed Martin Energy Systems plants had been designed and constructed to ASME Section VIII, Division 1 requirements. In the case of the normal stacking condition in the cylinder yards, the stacked cylinders under the dead weight create stresses, generally bending, especially at the regions where the high localized stresses occur. For a cylinder wall thickness of 0.250 inch, unless the defects, such as a gouge or cut of up to 1/16-inch depth are in the neighborhood of the stacking contacts, the cylinder meets Division 2 stress requirements.

The typical distance between the center of the wooden chock and the stiffener ring at the K-25 and Paducah sites is not more than 8 inches (4 inches from the surface of the ring to the closest point of the chock). The computations presented in Table 1 show that, the distance between the chock and the stiffner ring needs to be minimized and the chocks are better located on the head sides of the rings.

For the storage of cylinders by two tier stacking, Table 2 can be referenced. The distance between the upper and lower stiffener ring needs to be minimized. The lifting lugs should be carefully positioned to avoid contact with the cylinder shell. From computations with the file names S6L188FW, S6L250FW, and S6L312FW from Table 2, the localized stresses at the contact points are basically the bending stresses, and they are inversely proportion to the wall thickness considered.

For the lifting of breached cylinders, even though stresses are high at the region of the breach, cylinders can be lifted with the lifting lugs, straps, and Raygo Wagner if the remaining wall thickness is great or equal to 0.250 inch.

#### **REFERENCES**

American National Standards Institute, *Uranium Hexafluoride- Packaging for Transport,*  ANSI Standard Nl4.1 (1990).

American Society of Mechanical Engineers, *Boiler and Pressure Vessel Code,* Section VIII, Division I, and Section XI ( 1992).

Engineering Mechanics Research Corporation, Display III, NISA II (Version 91.0), Troy, Michigan, (1994).

Suresh, S., et al., "Environmentally Affected Near Threshold Fatigue Crack Growth in teels," Proceedings of the Fracture Mechanics 14th Symposium, Vol. 1, *Theory and Analysis* (1981).

U.S. Department of Energy, *Uranium Hexafluoride: A Manual of Good Handling Practices,* OR0-651 (Rev. 6), DE91015811, DOE Field Office, Oak Ridge, Tennessee.

## APPENDIX A. THE SURFACE FLAW OR GAUGE ON THE UF<sub>6</sub> CYLINDER

Method for Stress Intensity Factor  $(K<sub>i</sub>)$  Determination. The stress intensity factor for the flaw model can be calculated from the membrane and bending stresses, as provided in Article A-3000, Appendix A, ASME Section XI, at the flaw location using the following equation.

$$
K_l = S_m M_m (\pi a/Q)^{1/2} + S_b M_b (\pi a/Q)^{1/2}
$$

where

- $S_m$  = membrane stress (psi),
- $S_h^{\prime\prime}$  = bending stress (psi),
- $a =$ minor half-diameter (inches) of embedded flaw or flaw depth for surface flaw,
- $Q =$  flaw shape parameter,

 $M_m$  = correction factor for membrane stress, and

 $M<sub>b</sub>$  = correction factor for bending stress.

The crack extension resistance under conditions of crack tip plane strain, or plane strain fracture toughness,  $K_{IC}$ , is based on the lower bound of static initiation of critical  $K_I$  values measured as a function of temperature.  $K_{IC}$  of A-516, Gr.70 at the room temperature is obtained as 212.030 ksi $\sqrt{\ }$ <sub>in</sub> from Suresh et al. (1981). Since  $K_{1c}$  for the material A-516 Gr.55 is not available, that for A-516 Gr.70 was used.

Circumferential Stresses  $(S_{\theta})$  for the Surface Flaw. A sample problem of  $<$ S6L250FW $>$  is selected from Table B to calculate  $K<sub>1</sub>$ .

 $S_m = S_\theta^P = -1,400 \text{ psi}$  (@ 10 torr), and  $S_b = S_\theta^{DW} = 12,616 \text{ psi}$ .

The major half diameter of an elliptical flaw,  $\ell = 1.000$ ",  $S_v \leq Q$  100°F> = 30,000 psi,  $Q =$ 0.933,  $M_m = 1.33$ , and  $M_b = 1.00$ .

 $K_1 = (S_m M_m + S_b M_b)$  ( $\pi a/Q$ )<sup>%</sup> = (-1,400 \* 1.33 - 12,616 \* 1.00) (3.14 \* 0.0625 \*  $(0.933)^{0.5}$  = -6,197 psi  $\sqrt{\text{in}}$  =  $< (212,030 psi  $\sqrt{\text{in}}$ )$ 

Therefore, the flaw is not expected to propagate.

**Longitudinal Stresses.**  $S_m = S_l^P = -700$  psi (@ 10 torr) and  $S_b = S_l^{DW} = -24,607$  psi  $K_1 = (S_m M_m + S_b M_b)$  ( $\pi a/Q$ )<sup>1/2</sup> = (-700 \* 1.33 - 24,607 \* 1.00)  $(3.14 * 0.0625 * 0.933)^{0.5} = 10,931$  psi  $\sqrt{\text{in}}$ ) <<  $K_{IC}$  (212,030 psi  $\sqrt{\text{in}}$ ).

The flaw is not expected to propagate.

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